



STRUCTURAL DESIGN OF WOOD MEMBERS

This CodeMaster provides ten steps to guide the engineer in structurally designing typical wood framing members (columns, beams, joists, rafters, etc.) in accordance with the 2009 International Building Code (IBC), ASCE 7-05 Minimum Design Loads for Buildings and Other Structures (ASCE 7), and the 2005 National Design Specification[®] for Wood Construction (NDS[®]). The NDS includes a valuable commentary and an NDS Supplement which provides section properties and reference design values. Allowable Stress Design (ASD) is emphasized, however, Load and Resistance Factor Design (LRFD) is also included. This CodeMaster addresses the design of rectangular sawn lumber and structural glued laminated timbers (GLT). Wood structural panels, wood I-joists, structural composite lumber (LVL, PSL, LSL, OSB, etc.) and decking are not addressed.

Wood has a wide array of different material properties in separate orthogonal directions that are influenced by many factors. Recognizing wood's unique properties and proper application of adjustment factors is at the heart of timber design. Calculated stresses are denoted by lower case letters (such as f_b for calculated bending stress) and reference design values by upper case letters (such as F_b). The NDS denotes the adjusted design values with a "prime" marking (F_b' , F_c' , F_t' , F_v' , F_w' ; E' , E_{min}' , etc.), and it is these adjusted values that are the basis for design.

STEP 1 DETERMINE LOADS AND LOAD COMBINATIONS

Wood structural members such as beams and columns are subject to many types of loads, including gravity loads (e.g., dead, live, snow, and rain) and lateral loads (e.g., wind, earthquake, and soil). Gravity loads are determined by computing the structure's self-weight in addition to live and snow loads determined in accordance with IBC Sections 1607, 1608, and ASCE 7 Chapter 7. Wind loads are determined in accordance with IBC Section 1609 and ASCE 7 Chapter 6. Earthquake loads are determined in accordance with IBC Section 1613 and ASCE 7 Chapters 11 through 23, excluding Chapter 14. See the 2009 IBC Wind Overview CodeMaster and 2009 IBC Seismic Design CodeMaster for more information. Once the loads have been determined, the load path through the structure is analyzed to determine its effects such as bending moments, shear forces, compression forces, and tension forces on individual framing members. The load effects are then combined in accordance with the load combinations of IBC Section 1605.

SECRETS OF THE CODEMASTER: The self-weight of wood framing may be assumed to be 35 lb/ft² for most structural lumber at typical equilibrium moisture contents. NDS Supplement Table 1B provides unit weights for common member sizes. However, published dead loads assumed for certain applications (e.g., residential) often already include self-weight of sawn lumber framing members.

SECRETS OF THE CODEMASTER: Designers have a choice of using ASD or LRFD load combinations. While ASD is more conventional, designs using LRFD loads are easily adaptable with simple conversion factors and with most adjustments remaining unchanged.

STEP 2 DETERMINE AXIAL, SHEAR, AND BENDING FORCES

Using unit loads from STEP 1 in conjunction with tracing the load path, member bending, shear, compression, and tension forces are determined. Computing member tributary area is critical to load path analysis and is also necessary in determining any applicable live load reductions. Tributary area live load reductions are determined in accordance with IBC Section 1607.5 (floor live) and Section 1607.11.2 (roof live). Where applied beam loads occur near the member's support, the design shear force may be reduced per NDS Section 3.4.3.1.

Figure 1. Roof or Floor Framing Plan

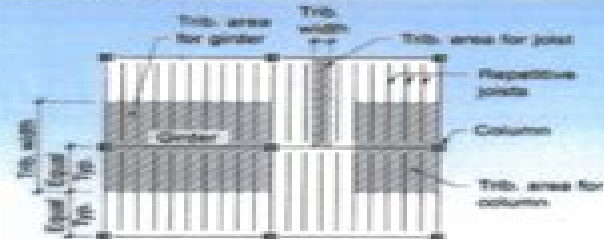
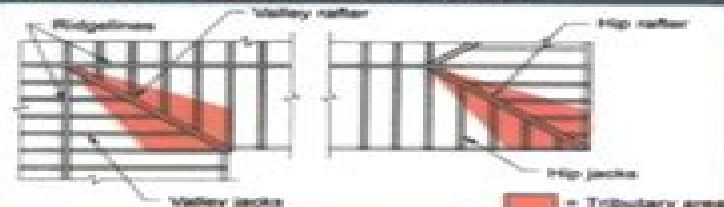


Figure 2. Tributary Areas of Valley and Hip Rafters



SECRETS OF THE CODEMASTER: For sloping bending members such as roof rafters, roof live loads and snow loads are defined as horizontal-projected plane loads, and it is often acceptable to simply use horizontal span lengths for the bending member analysis. While this simplification ignores the increased length of the rafter relative to a horizontal projection, it also ignores the additional bending and compression load capacity provided by the roof sheathing and the reduced loads using a sloped rafter length versus the horizontal projection.

Figure 3. Typical Rafter Span Simplification



STEP 3 SELECT MATERIALS TO BE USED

Selection of sawn lumber member size and wood species group is largely driven by regional availability. For a given location, only a few species groups will generally be available. Most common are the following softwood species groups:

Douglas fir-larch
Southern pine

Hem-fir
Spruce-pine-fir

Most typically specified are dressed sizes which are $\frac{1}{8}$ -inch to $\frac{1}{4}$ -inch smaller than specified nominal sizes. The range of dressed sizes and their section properties are in NDS Supplement Table 1B. Dimensional size terminology defines thickness (breadth) as the narrow dimension and width (depth) as the largest dimension.

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 - The Rise of Digital Reading Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
 - Advantages of eBooks Over Traditional Books
2. Identifying Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
 - Exploring Different Genres
 - Considering Fiction vs. Non-Fiction
 - Determining Your Reading Goals
3. Choosing the Right eBook Platform
 - Popular eBook Platforms
 - Features to Look for in an Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
 - User-Friendly Interface
4. Exploring eBook Recommendations from Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds

- Personalized Recommendations
 - Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds User Reviews and Ratings
 - Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds and Bestseller Lists
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 - Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds eBook Subscription Services
 - Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds Budget-Friendly Options
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- ePub, PDF, MOBI, and More
 - Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds Compatibility with Devices
 - Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds Enhanced eBook Features
7. Enhancing Your Reading Experience
- Adjustable Fonts and Text Sizes of Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
 - Highlighting and Note-Taking Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
 - Interactive Elements Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
8. Staying Engaged with Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
- Joining Online Reading Communities
 - Participating in Virtual Book Clubs
 - Following Authors and Publishers Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
9. Balancing eBooks and Physical Books Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
- Benefits of a Digital Library
 - Creating a Diverse Reading Collection Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
10. Overcoming Reading Challenges
- Dealing with Digital Eye Strain
 - Minimizing Distractions
 - Managing Screen Time
11. Cultivating a Reading Routine Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
- Setting Reading Goals Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
 - Carving Out Dedicated Reading Time
12. Sourcing Reliable Information of Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds

- Fact-Checking eBook Content of Codemaster Structural Wood Design Asdlrfd 2012 Ibc Asce 7 10 2012 Nds
- Distinguishing Credible Sources

13. Promoting Lifelong Learning

- Utilizing eBooks for Skill Development
- Exploring Educational eBooks

14. Embracing eBook Trends

- Integration of Multimedia Elements
- Interactive and Gamified eBooks

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